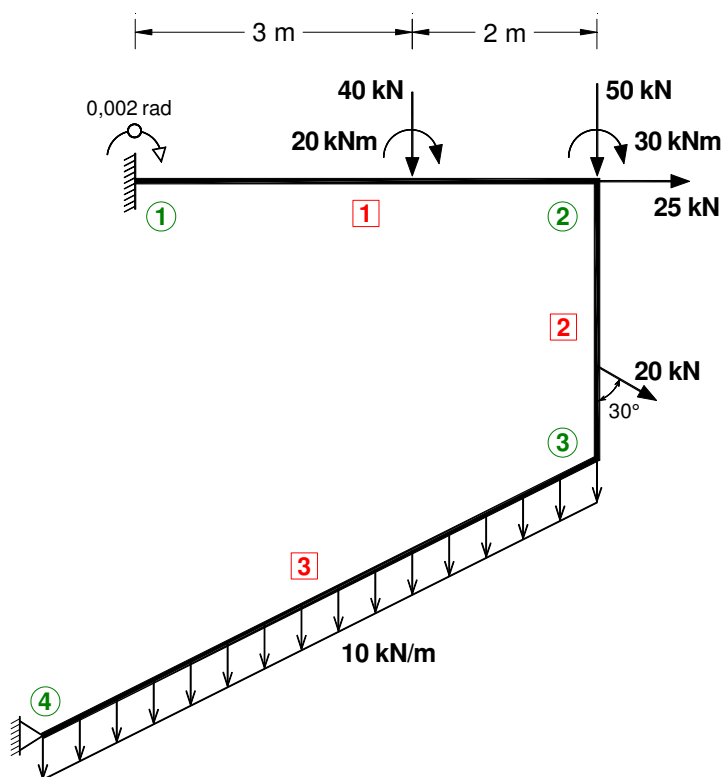


LICENCIATURA EM ENGENHARIA CIVIL

TEORIA DE ESTRUTURAS

MÉTODO DOS DESLOCAMENTOS

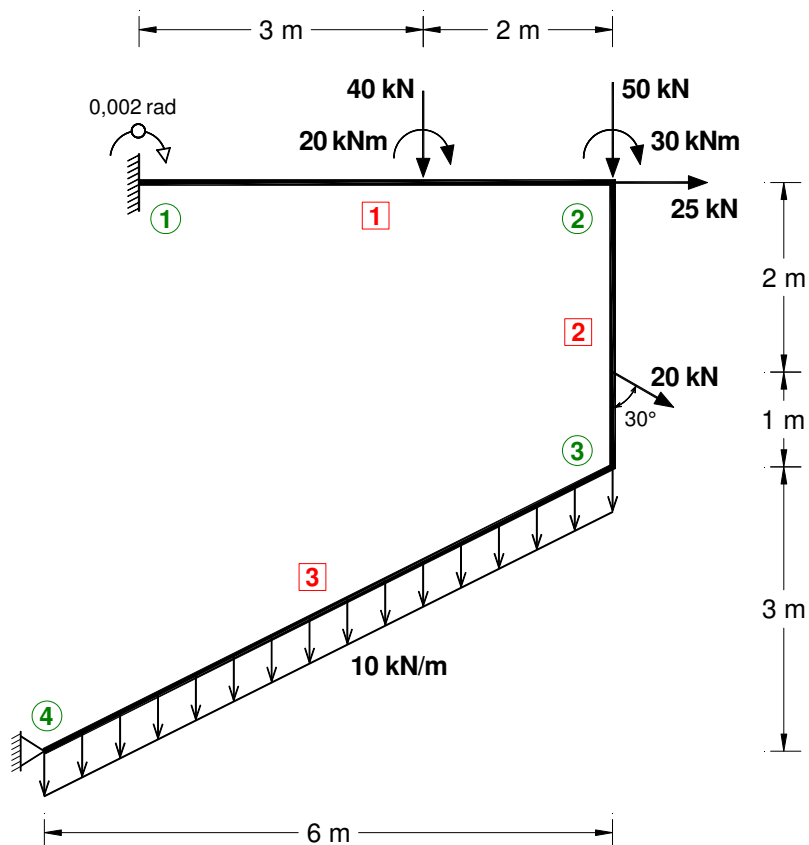


ESTRUTURA CONTÍNUA

ISABEL ALVIM TELES

EXERCÍCIO PROPOSTO

Considere a estrutura representada na figura, realizada em betão com $E = 25 \text{ GPa}$ e em que todas as barras têm secção $0,30 \text{ m} \times 0,40 \text{ m}$.



Responda às alíneas aplicando o Método dos Deslocamentos.

- Determine os deslocamentos dos nós da estrutura;
- Determine as reações nos apoios;
- Trace os diagramas de esforços da estrutura.

RESOLUÇÃO

• MATRIZ DE RIGIDEZ ELEMENTAR – SISTEMA DE EIXOS LOCAL

$$[K_L] = \begin{bmatrix} \frac{EA}{L} & 0 & 0 & -\frac{EA}{L} & 0 & 0 \\ 0 & \frac{12EI}{L^3} & \frac{6EI}{L^2} & 0 & -\frac{12EI}{L^3} & \frac{6EI}{L^2} \\ 0 & \frac{6EI}{L^2} & \frac{4EI}{L} & 0 & -\frac{6EI}{L^2} & \frac{2EI}{L} \\ -\frac{EA}{L} & 0 & 0 & \frac{EA}{L} & 0 & 0 \\ 0 & -\frac{12EI}{L^3} & -\frac{6EI}{L^2} & 0 & \frac{12EI}{L^3} & -\frac{6EI}{L^2} \\ 0 & \frac{6EI}{L^2} & \frac{2EI}{L} & 0 & -\frac{6EI}{L^2} & \frac{4EI}{L} \end{bmatrix}$$

• MATRIZ DE TRANSFORMAÇÃO

$$[T] = \begin{bmatrix} \cos \alpha & \sin \alpha & 0 & 0 & 0 & 0 \\ -\sin \alpha & \cos \alpha & 0 & 0 & 0 & 0 \\ 0 & 0 & 1 & 0 & 0 & 0 \\ 0 & 0 & 0 & \cos \alpha & \sin \alpha & 0 \\ 0 & 0 & 0 & -\sin \alpha & \cos \alpha & 0 \\ 0 & 0 & 0 & 0 & 0 & 1 \end{bmatrix}$$

• MATRIZ DE TRANSFORMAÇÃO TRANSPOSTA

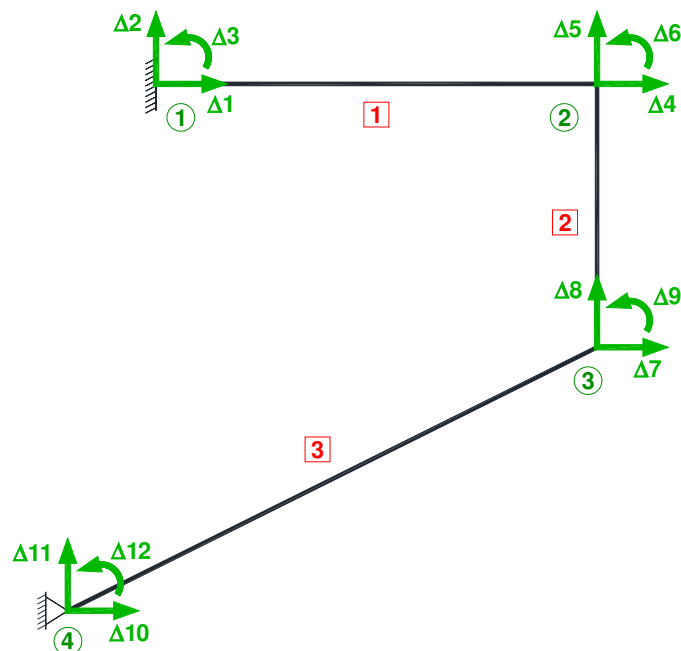
$$[T]^T = \begin{bmatrix} \cos \alpha & -\sin \alpha & 0 & 0 & 0 & 0 \\ \sin \alpha & \cos \alpha & 0 & 0 & 0 & 0 \\ 0 & 0 & 1 & 0 & 0 & 0 \\ 0 & 0 & 0 & \cos \alpha & -\sin \alpha & 0 \\ 0 & 0 & 0 & \sin \alpha & \cos \alpha & 0 \\ 0 & 0 & 0 & 0 & 0 & 1 \end{bmatrix}$$

• MATRIZ DE RIGIDEZ ELEMENTAR – SISTEMA DE EIXOS GLOBAL

$$[K_G] = [T]^T [K_L] [T]$$

ALÍNEA a)

• GRAUS DE LIBERDADE DA ESTRUTURA (SISTEMA GLOBAL)



• MATRIZ DE RIGIDEZ LOCAL, MATRIZ DE TRANSFORMAÇÃO E MATRIZ DE RIGIDEZ GLOBAL

BARRA 1 Nós 1 - 2 Graus de liberdade (sistema global): $\Delta 1, \Delta 2, \Delta 3$ - $\Delta 4, \Delta 5, \Delta 6$

$E = 25 \times 10^6 \text{ kPa}$	$b = 0.3 \text{ m}$	$A = 0.12 \text{ m}^2$	$\alpha = 0^\circ \Rightarrow$	$\cos \alpha = 1$
$L = 5 \text{ m}$	$h = 0.4 \text{ m}$	$I = 0.0016 \text{ m}^4$		$\sin \alpha = 0$

$$\left[K_L^1 \right] = \begin{bmatrix} 600\,000 & 0 & 0 & -600\,000 & 0 & 0 \\ 0 & 3\,840 & 9\,600 & 0 & -3\,840 & 9\,600 \\ 0 & 9\,600 & 32\,000 & 0 & -9\,600 & 16\,000 \\ \hline -600\,000 & 0 & 0 & 600\,000 & 0 & 0 \\ 0 & -3\,840 & -9\,600 & 0 & 3\,840 & -9\,600 \\ 0 & 9\,600 & 16\,000 & 0 & -9\,600 & 32\,000 \end{bmatrix}$$

$$\left[T^1 \right] = \begin{bmatrix} 1 & 0 & 0 & 0 & 0 & 0 \\ 0 & 1 & 0 & 0 & 0 & 0 \\ 0 & 0 & 1 & 0 & 0 & 0 \\ \hline 0 & 0 & 0 & 1 & 0 & 0 \\ 0 & 0 & 0 & 0 & 1 & 0 \\ 0 & 0 & 0 & 0 & 0 & 1 \end{bmatrix}$$

$$\left[K_G^1 \right] = \left[K_L^1 \right] = \begin{matrix} & \begin{matrix} 1 & 2 & 3 & 4 & 5 & 6 \end{matrix} \\ \begin{matrix} 1 \\ 2 \\ 3 \\ 4 \\ 5 \\ 6 \end{matrix} & \begin{bmatrix} 600\,000 & 0 & 0 & -600\,000 & 0 & 0 \\ 0 & 3\,840 & 9\,600 & 0 & -3\,840 & 9\,600 \\ 0 & 9\,600 & 32\,000 & 0 & -9\,600 & 16\,000 \\ \hline -600\,000 & 0 & 0 & 600\,000 & 0 & 0 \\ 0 & -3\,840 & -9\,600 & 0 & 3\,840 & -9\,600 \\ 0 & 9\,600 & 16\,000 & 0 & -9\,600 & 32\,000 \end{bmatrix} \end{matrix}$$

BARRA 2 Nós **2 - 3** Graus de liberdade (sistema global): **Δ4, Δ5, Δ6 - Δ7, Δ8, Δ9**

$E = 25 \times 10^6 \text{ kPa}$	$b = 0.3 \text{ m}$	$A = 0.12 \text{ m}^2$	$\alpha = -90^\circ \Rightarrow$	$\cos \alpha = 0$
$L = 3 \text{ m}$	$h = 0.4 \text{ m}$	$I = 0.0016 \text{ m}^4$		$\sin \alpha = -1$

$$[K_L^2] = \begin{bmatrix} 1000\,000 & 0 & 0 & -1000\,000 & 0 & 0 \\ 0 & 17\,777,78 & 26\,666,67 & 0 & -17\,777,78 & 26\,666,67 \\ 0 & 26\,666,67 & 53\,333,33 & 0 & -26\,666,67 & 26\,666,67 \\ -1000\,000 & 0 & 0 & 1000\,000 & 0 & 0 \\ 0 & -17\,777,78 & -26\,666,67 & 0 & 17\,777,78 & -26\,666,67 \\ 0 & 26\,666,67 & 26\,666,67 & 0 & -26\,666,67 & 53\,333,33 \end{bmatrix} \quad [T^2] = \begin{bmatrix} 0 & -1 & 0 & 0 & 0 & 0 \\ 1 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 1 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & -1 & 0 \\ 0 & 0 & 0 & 1 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 1 \end{bmatrix}$$

$$[K_G^2] = [T^2]^T [K_L^2] [T^2] = \begin{bmatrix} 17\,777,78 & 0 & 26\,666,67 & -17\,777,78 & 0 & 26\,666,67 \\ 0 & 1000000 & 0 & 0 & -1000000 & 0 \\ 26\,666,67 & 0 & 53\,333,33 & -26\,666,67 & 0 & 26\,666,67 \\ -17\,777,78 & 0 & -26\,666,67 & 17\,777,78 & 0 & -26\,666,67 \\ 0 & -1000000 & 0 & 0 & 1000000 & 0 \\ 26\,666,67 & 0 & 26\,666,67 & -26\,666,67 & 0 & 53\,333,33 \end{bmatrix}$$

BARRA 3 Nós **3 - 4** Graus de liberdade (sistema global): **Δ7, Δ8, Δ9 - Δ10, Δ11, Δ12**

$E = 25 \times 10^6 \text{ kPa}$	$b = 0.3 \text{ m}$	$A = 0.12 \text{ m}^2$	$\alpha = -153,435^\circ \Rightarrow$	$\cos \alpha = -0.8944$
$L = 3\sqrt{5} \text{ m}$	$h = 0.4 \text{ m}$	$I = 0.0016 \text{ m}^4$		$\sin \alpha = -0.4472$

$$[K_L^3] = \begin{bmatrix} 447\,213,60 & 0 & 0 & -447\,213,60 & 0 & 0 \\ 0 & 1\,590,09 & 5\,333,33 & 0 & -1\,590,09 & 5\,333,33 \\ 0 & 5\,333,33 & 23\,851,39 & 0 & -5\,333,33 & 11\,925,70 \\ -447\,213,60 & 0 & 0 & 447\,213,60 & 0 & 0 \\ 0 & -1\,590,09 & -5\,333,33 & 0 & 1\,590,09 & -5\,333,33 \\ 0 & 5\,333,33 & 11\,925,70 & 0 & -5\,333,33 & 23\,851,39 \end{bmatrix} \quad [T^3] = \begin{bmatrix} -0,8944 & -0,4472 & 0 & 0 & 0 & 0 \\ 0,4472 & -0,8944 & 0 & 0 & 0 & 0 \\ 0 & 0 & 1 & 0 & 0 & 0 \\ 0 & 0 & 0 & -0,8944 & -0,4472 & 0 \\ 0 & 0 & 0 & 0,4472 & -0,8944 & 0 \\ 0 & 0 & 0 & 0 & 0 & 1 \end{bmatrix}$$

$$[K_G^3] = [T^3]^T [K_L^3] [T^3] = \begin{bmatrix} 358\,088,89 & 178\,249,40 & 2\,385,14 & -358\,088,89 & -178\,249,40 & 2\,385,14 \\ 178\,249,40 & 90\,714,79 & -4\,770,28 & -178\,249,40 & -90\,714,79 & -4\,770,28 \\ 2\,385,14 & -4\,770,28 & 23\,851,39 & -2\,385,14 & 4\,770,28 & 11\,925,70 \\ -358\,088,89 & -178\,249,40 & -2\,385,14 & 358\,088,89 & 178\,249,40 & -2\,385,14 \\ -178\,249,40 & -90\,714,79 & 4\,770,28 & 178\,249,40 & 90\,714,79 & 4\,770,28 \\ 2\,385,14 & -4\,770,28 & 11\,925,70 & -2\,385,14 & 4\,770,28 & 23\,851,39 \end{bmatrix}$$

• VETOR DE SOLICITAÇÃO

Solicitação aplicada nos nós (sistema global)

	Nó 2
4	25
5	- 50
6	- 30

Solicitação aplicada nas barras – Forças equivalentes nodais

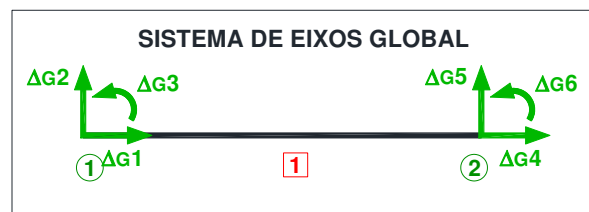
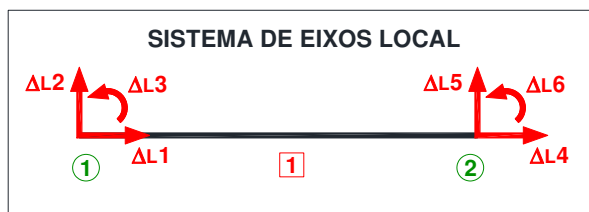
$$F_{Global} = T^T \cdot F_{Local}$$

BARRA 1

Nós 1 - 2

Graus de liberdade - sistema local: $\Delta L1, \Delta L2, \Delta L3$ - $\Delta L4, \Delta L5, \Delta L6$

Graus de liberdade - sistema global: $\Delta G1, \Delta G2, \Delta G3$ - $\Delta G4, \Delta G5, \Delta G6$



$P_y = -40 \text{ kN}$

$$F_{//eq} = 0$$

$$F_{\perp eq} = \frac{P_y \cdot b^2}{L^3} (L + 2a) = \frac{-40 \times 2^2}{5^3} (5 + 2 \times 3) = -14,08 \text{ kN}$$

$$F_{M-esq} = \frac{P_y \cdot a \cdot b^2}{L^2} = \frac{-40 \times 3 \times 2^2}{5^2} = -19,2 \text{ kNm}$$

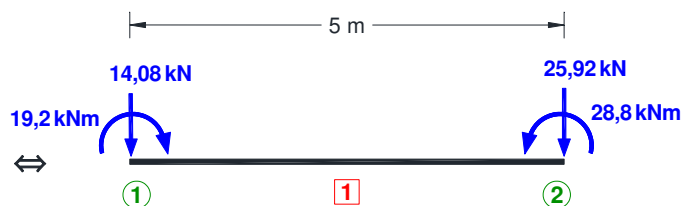
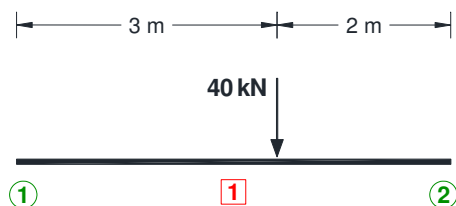
$$F_{//dir} = 0$$

$$F_{\perp dir} = \frac{P_y \cdot a^2}{L^3} (L + 2b) = \frac{-40 \times 3^2}{5^3} (5 + 2 \times 2) = -25,92 \text{ kN}$$

$$F_{M-dir} = -\frac{P_y \cdot a^2 \cdot b}{L^2} = -\frac{-40 \times 3^2 \times 2}{5^2} = 28,8 \text{ kNm}$$

$P_y = -40 \text{ kN}$
$F_{L-EN} - LOCAL$

F_1	0
F_2	-14,08
F_3	-19,2
F_4	0
F_5	-25,92
F_6	28,8



$M_z = -20 \text{ kNm}$

$$F_{//\text{esq}} = 0$$

$$F_{\perp\text{esq}} = -\frac{6 M_z}{L^3} a \cdot b = -\frac{6 \times (-20)}{5^3} \times 3 \times 2 = 5,76 \text{ kN}$$

$$F_{M\text{-esq}} = -\frac{M_z \cdot b}{L} \left(2 - \frac{3b}{L} \right) = -\frac{-20 \times 2}{5} \left(2 - \frac{3 \times 2}{5} \right) = 6,4 \text{ kNm}$$

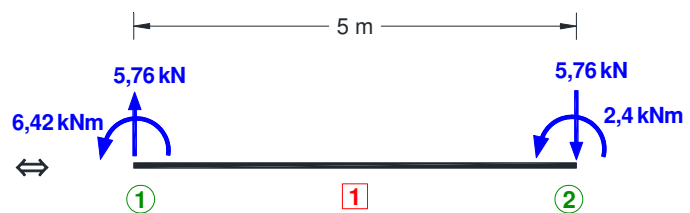
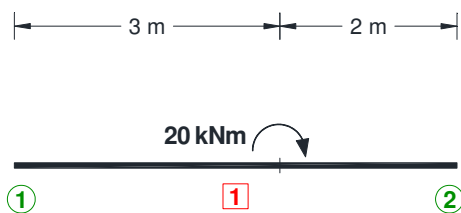
$$F_{//\text{dir}} = 0$$

$$F_{\perp\text{dir}} = \frac{6 M_z}{L^3} a \cdot b = \frac{6 \times (-20)}{5^3} \times 3 \times 2 = -5,76 \text{ kN}$$

$$F_{M\text{-dir}} = -\frac{M_z \cdot a}{L} \left(2 - \frac{3a}{L} \right) = -\frac{-20 \times 3}{5} \left(2 - \frac{3 \times 3}{5} \right) = 2,4 \text{ kNm}$$

$M_z = -20 \text{ kNm}$
 $F_{L-EN} - \text{LOCAL}$

F_1	0
F_2	5,76
F_3	6,4
F_4	0
F_5	-5,76
F_6	2,4



$P_y = -40 \text{ kN} + M_z = -20 \text{ kNm}$
 $F_{L-EN} - \text{LOCAL}$

F_1	0 + 0
F_2	-14,08 + 5,76
F_3	-19,2 + 6,4
F_4	0 + 0
F_5	-25,92 - 5,76
F_6	28,8 + 2,4

Barra 1 - Nós 1 - 2
 $F_{L-EN} - \text{LOCAL}$

F_1	0
F_2	-8,32
F_3	-12,8
F_4	0
F_5	-31,68
F_6	31,2

É fácil relacionar o sistema de eixos local com o global:

$\Delta L1 \Leftrightarrow \Delta G1$
 $\Delta L2 \Leftrightarrow \Delta G2$
 $\Delta L3 \Leftrightarrow \Delta G3$
 $\Delta L4 \Leftrightarrow \Delta G4$
 $\Delta L5 \Leftrightarrow \Delta G5$
 $\Delta L6 \Leftrightarrow \Delta G6$

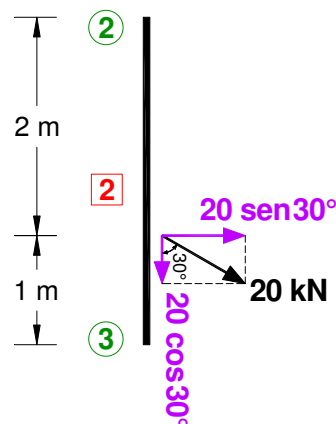
Barra 1 - Nós 1 - 2
 $F_{G-EN} - \text{GLOBAL}$

F_1	0
F_2	-8,32
F_3	-12,8
F_4	0
F_5	-31,68
F_6	31,2

BARRA 2 Nós **2 - 3**

Graus de liberdade - sistema local: $\Delta L1, \Delta L2, \Delta L3$ - $\Delta L4, \Delta L5, \Delta L6$

Graus de liberdade - sistema global: $\Delta G1, \Delta G2, \Delta G3$ - $\Delta G4, \Delta G5, \Delta G6$



Força 20 kN $\Rightarrow$
$$\begin{cases} P_y = 20 \text{ sen } 30^\circ \text{ kN} & (\perp \text{ à barra}) \\ P_x = 20 \text{ cos } 30^\circ \text{ kN} & (// \text{ à barra}) \end{cases}$$

$P_y = 20 \text{ sen } 30^\circ \text{ kN}$

$$F_{//\text{esq}} = 0$$

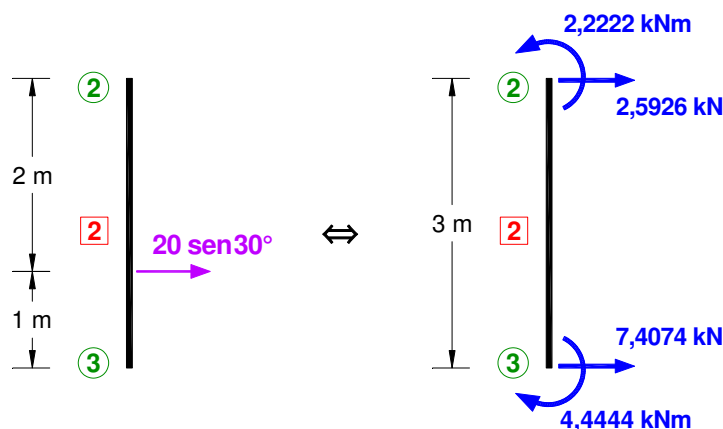
$$F_{\perp\text{esq}} = \frac{P_y \cdot b^2}{L^3} (L + 2a) = \frac{20 \text{ sen } 30^\circ \times 1^2}{3^3} (3 + 2 \times 2) = 2,5926 \text{ kN}$$

$$F_{M\text{-esq}} = \frac{P_y \cdot a \cdot b^2}{L^2} = \frac{20 \text{ sen } 30^\circ \times 2 \times 1^2}{3^2} = 2,2222 \text{ kNm}$$

$$F_{//\text{dir}} = 0$$

$$F_{\perp\text{dir}} = \frac{P_y \cdot a^2}{L^3} (L + 2b) = \frac{20 \text{ sen } 30^\circ \times 2^2}{3^3} (3 + 2 \times 1) = 7,4074 \text{ kN}$$

$$F_{M\text{-dir}} = -\frac{P_y \cdot a^2 \cdot b}{L^2} = -\frac{20 \text{ sen } 30^\circ \times 2^2 \times 1}{3^2} = -4,4444 \text{ kNm}$$

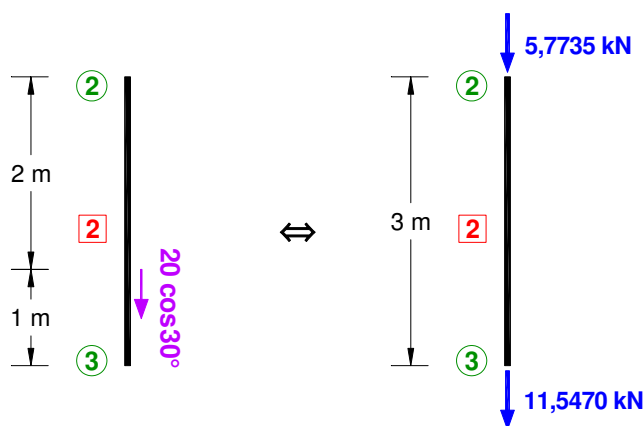


$P_y = 20 \text{ sen } 30^\circ \text{ kN}$

$F_{L-EN} - \text{LOCAL}$

F_1	0
F_2	2,5926
F_3	2,2222
F_4	0
F_5	7,4074
F_6	-4,4444

$P_x = 20 \cos 30^\circ \text{ kN}$	$F_{//\text{esq}} = \frac{P_x \cdot b}{L} = \frac{20 \cos 30^\circ \times 1}{3} = 5,7735 \text{ kN}$
	$F_{\perp\text{esq}} = 0$
	$F_{M\text{-esq}} = 0$
	$F_{//\text{dir}} = \frac{P_x \cdot a}{L} = \frac{20 \cos 30^\circ \times 2}{3} = 11,5470 \text{ kN}$
	$F_{\perp\text{dir}} = 0$
	$F_{M\text{-dir}} = 0$



$P_x = 20 \cos 30^\circ \text{ kN}$
$F_{L-EN} - \text{LOCAL}$

F_1	5,7735
F_2	0
F_3	0
F_4	11,5470
F_5	0
F_6	0

$P_y = 20 \cos 30^\circ \text{ kN} + P_x = 20 \cos 30^\circ \text{ kN}$
$F_{L-EN} - \text{LOCAL}$

F_1	0 + 5,7735
F_2	2,5926 + 0
F_3	2,2222 + 0
F_4	0 + 11,5470
F_5	7,4074 + 0
F_6	-4,4444 + 0

Barra 2 - Nós 2 - 3
$F_{L-EN} - \text{LOCAL}$

F_1	5,7735
F_2	2,5926
F_3	2,2222
F_4	11,5470
F_5	7,4074
F_6	-4,4444

É fácil relacionar o sistema de eixos local com o global:

$\Delta L1 \Leftrightarrow -\Delta G5$	$\Delta G4 \Leftrightarrow \Delta L2$
$\Delta L2 \Leftrightarrow \Delta G4$	$\Delta G5 \Leftrightarrow -\Delta L1$
$\Delta L3 \Leftrightarrow \Delta G6$	$\Delta G6 \Leftrightarrow \Delta L3$
$\Delta L4 \Leftrightarrow -\Delta G8$	$\Delta G7 \Leftrightarrow \Delta L5$
$\Delta L5 \Leftrightarrow \Delta G7$	$\Delta G8 \Leftrightarrow -\Delta L4$
$\Delta L6 \Leftrightarrow \Delta G9$	$\Delta G9 \Leftrightarrow \Delta L6$

Barra 2 - Nós 2 - 3
$F_{G-EN} - \text{GLOBAL}$

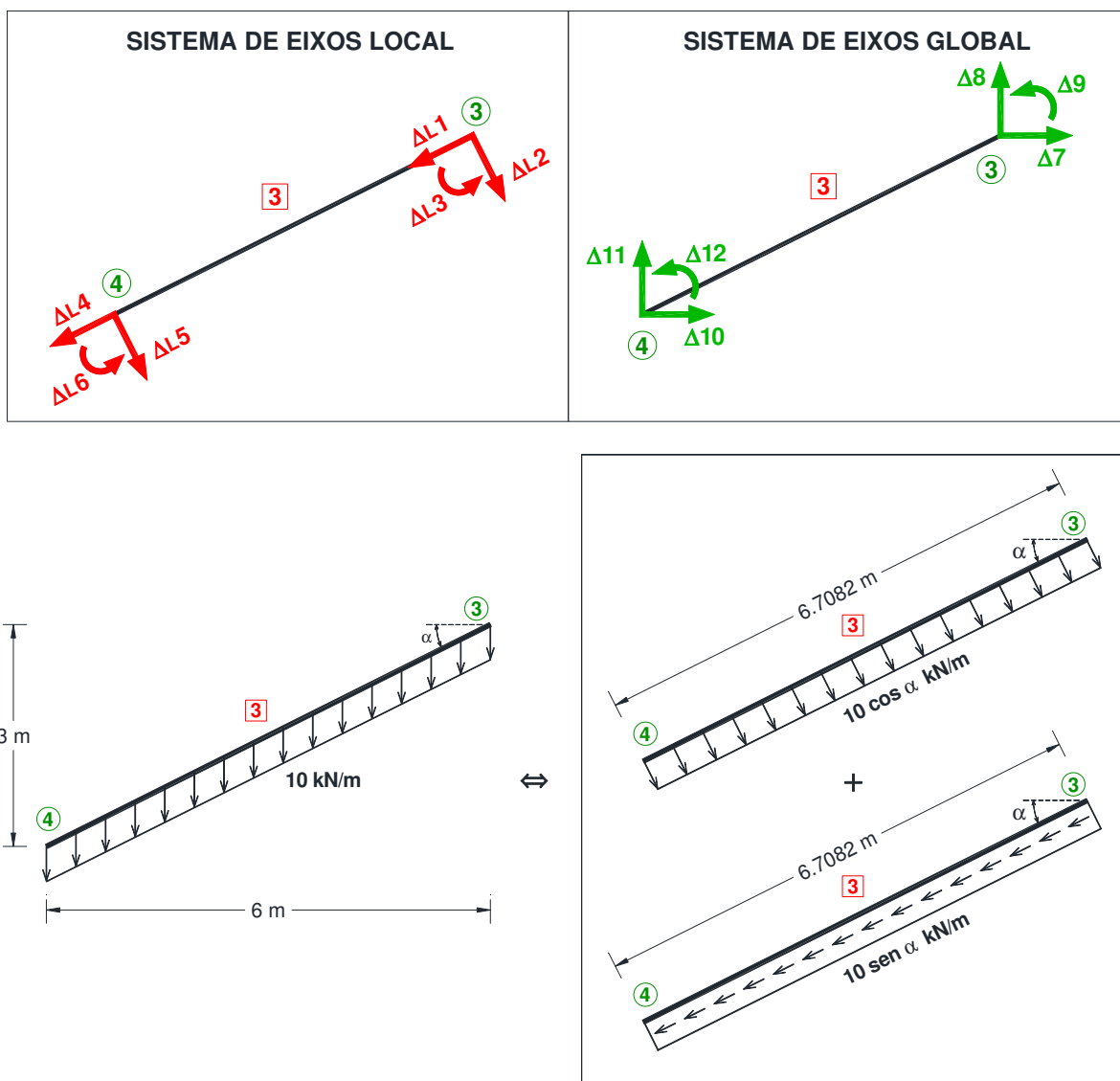
F_4	2,5926
F_5	-5,7735
F_6	2,2222
F_7	7,4074
F_8	-11,5470
F_9	-4,4444

BARRA 3

Nós 3 - 4

Graus de liberdade - sistema local: $\Delta L1, \Delta L2, \Delta L3$ - $\Delta L4, \Delta L5, \Delta L6$

Graus de liberdade - sistema global: $\Delta G7, \Delta G8, \Delta G9$ - $\Delta G10, \Delta G11, \Delta G12$



Comprimento da barra: $L = 3\sqrt{5} = 6,7082 \text{ m}$

$$\alpha = \arctg \frac{3}{6} \Rightarrow \alpha = 26,565^\circ \Rightarrow \begin{cases} \cos \alpha = 0,8944 \\ \sin \alpha = 0,4472 \end{cases}$$

$$10 \cos \alpha = 8,9443 \text{ kN/m}$$

$$10 \sin \alpha = 4,4721 \text{ kN/m}$$

TEORIA DE ESTRUTURAS

ISABEL ALVIM TELES

$p_y = -10 \cos \alpha \text{ kN/m}$

$$F_{//esq} = 0$$

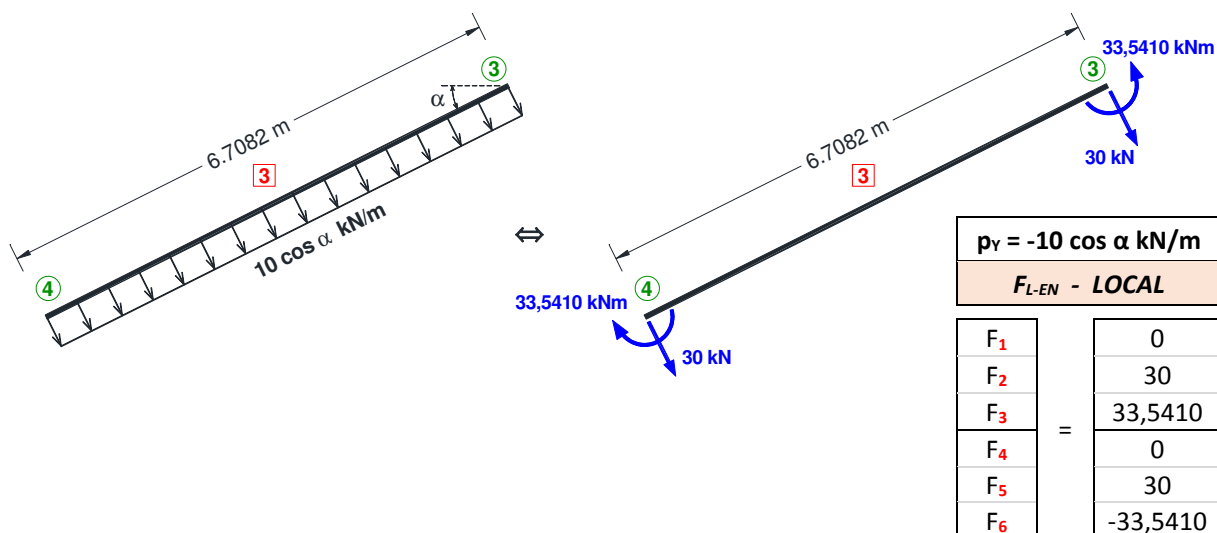
$$F_{\perp esq} = \frac{p_y \cdot L}{2} = \frac{-10 \cos \alpha \times 3\sqrt{5}}{2} = -30 \text{ kN}$$

$$F_{M-esq} = \frac{p_y \cdot L^2}{12} = \frac{-10 \cos \alpha \times (3\sqrt{5})^2}{12} = -33,5410 \text{ kNm}$$

$$F_{//dir} = 0$$

$$F_{\perp dir} = \frac{p_y \cdot L}{2} = \frac{-10 \cos \alpha \times 3\sqrt{5}}{2} = -30 \text{ kN}$$

$$F_{M-dir} = -\frac{p_y \cdot L^2}{12} = -\frac{-10 \cos \alpha \times (3\sqrt{5})^2}{12} = 33,5410 \text{ kNm}$$



$p_x = -10 \sin \alpha \text{ kN/m}$

$$F_{//esq} = \frac{p_x \cdot L}{2} = \frac{-10 \sin \alpha \times 3\sqrt{5}}{2} = -15 \text{ kN}$$

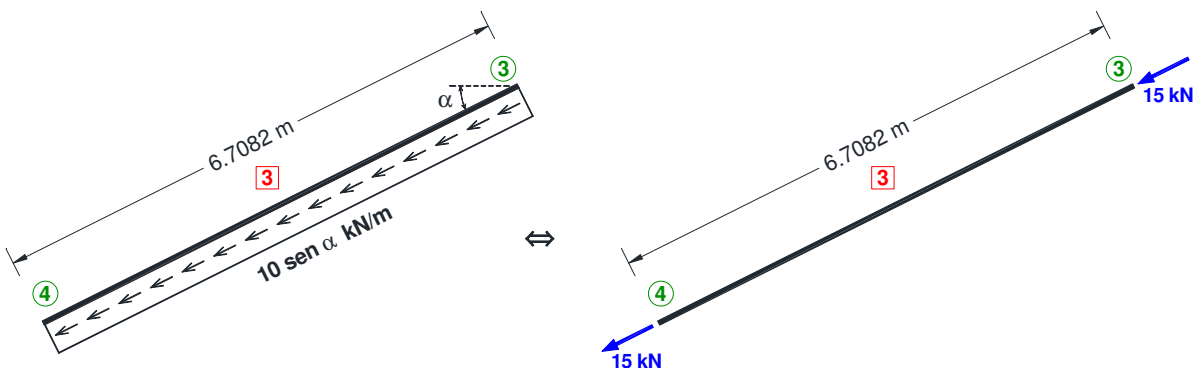
$$F_{\perp esq} = 0$$

$$F_{M-esq} = 0$$

$$F_{//dir} = \frac{p_x \cdot L}{2} = \frac{-10 \sin \alpha \times 3\sqrt{5}}{2} = -15 \text{ kN}$$

$$F_{\perp dir} = 0$$

$$F_{M-dir} = 0$$



$p_y = -10 \cos \alpha \text{ kN/m} + p_x = -10 \cos \alpha \text{ kN/m}$		Barra 3 - Nós 3 - 4	
F_{L-EN} - LOCAL		F_{L-EN} - LOCAL	
F_1	$0 + 15$	F_1	15
F_2	$30 + 0$	F_2	30
F_3	$33,5410 + 0$	F_3	$33,5410$
F_4	$0 + 15$	F_4	15
F_5	$30 + 0$	F_5	30
F_6	$-33,5410 + 0$	F_6	$-33,5410$

$$F_{Global} = T^T \cdot F_{Local}$$

F_{G-EN}		T^T		F_{L-EN}		Barra 3 - Nós 3 - 4	
F_{G-EN} - GLOBAL						F_{G-EN} - GLOBAL	
7 F_7	-0,8944	0,4472	0	0	0	0	15
8 F_8	-0,4472	-0,8944	0	0	0	0	30
9 F_9	0	0	1	0	0	0	33,5410
10 F_{10}	0	0	0	-0,8944	0,4472	0	15
11 F_{11}	0	0	0	-0,4472	-0,8944	0	30
12 F_{12}	0	0	0	0	0	1	-33,5410

Vetor solicitação - sistema de eixos global											
1	0	1	0	2	- 8,32	2	- 8,32	3	- 12,8	3	- 12,8
4	25 + 0 + 2,5926	4	27,5926	5	- 50 - 31,68 - 5,7735	5	- 87,4535	6	- 30 + 31,20 + 2,2222	6	3,4222
7	7,4074 + 0	7	7,4074	8	- 11,5470 + 33,5410	8	- 45,088	9	- 4,4444 + 33,5410	9	29,0966
10	0	10	0	11	- 33,5410	11	- 33,5410	12	- 33,5410	12	- 33,5410

<u>Vetor das reações</u> sistema de eixos global		<u>Vetor dos deslocamentos</u> sistema de eixos global	
1	R1	1	0
2	R2	2	0
3	R3	3	- 0,002
4	0	4	$\Delta 4$
5	0	5	$\Delta 5$
6	0	6	$\Delta 6$
7	0	7	$\Delta 7$
8	0	8	$\Delta 8$
9	0	9	$\Delta 9$
10	R10	10	0
11	R11	11	0
12	0	12	$\Delta 12$

- **MATRIZ DE RIGIDEZ GLOBAL DA ESTRUTURA**

$$[K_G]^{Est} \cdot [\Delta_G] = [F_G] + [R_G]$$

$$\begin{bmatrix} 4 & 5 & 6 & 7 & 8 & 9 & 12 & 1 & 2 & 3 & 10 & 11 \\ 4 & & & & & & & & & & & \\ 5 & & & & & & & & & & & \\ 6 & & & & & & & & & & & \\ 7 & & & K_{LL} & & & & & K_{LF} & & & \\ 8 & & & & & & & & & & & \\ 9 & & & & & & & & & & & \\ 12 & & & & & & & & & & & \\ 1 & & & & & & & & & & & \\ 2 & & & & & & & & & & & \\ 3 & & & K_{FL} & & & & & K_{FF} & & & \\ 10 & & & & & & & & & & & \\ 11 & & & & & & & & & & & \end{bmatrix} * \begin{bmatrix} \Delta_{Li} \\ \vdots \\ \Delta_F \end{bmatrix} = \begin{bmatrix} F_{Li} \\ \vdots \\ F_F \end{bmatrix} + \begin{bmatrix} 0 \\ \vdots \\ R_F \end{bmatrix}$$

$$\left\{ \begin{array}{l} [\mathbf{K}_{LL}] \cdot [\Delta_{Li}] + [\mathbf{K}_{LF}] \cdot [\Delta_F] = [\mathbf{F}_L] \\ [\mathbf{K}_{FL}] \cdot [\Delta_{Li}] + [\mathbf{K}_{FF}] \cdot [\Delta_F] = [\mathbf{F}_F] + [\mathbf{R}_F] \end{array} \right. \Rightarrow [\Delta_{Li}]$$

Na página seguinte estão representados o sistema $[K_G]^{\text{Est}}$. $[\Delta_G] = [F_G] + [R_G]$ na forma inicial, com os graus de liberdade ordenados de forma crescente do 1 ao 12, e na forma rearranjada, separando graus de liberdade sem restrições de deslocamento (livres): 4, 5, 6, 7, 8, 9 e 12 e graus de liberdade com restrições de deslocamento (fixos): 1, 2, 3, 10 e 11.

TEORIA DE ESTRUTURAS

ISABEL ALVIM TELES

K _G - Estrutura											
1	2	3	4	5	6	7	8	9	10	11	12
1	600 000	0	0	-600 000	0	0	0	0	0	0	0
2	0	3840	9600	0	-3840	9600	0	0	0	0	0
3	0	9600	32 000	0	-9600	16 000	0	0	0	0	0
4	-600 000	0	0	617 777,78	0	26 666,67	-17 777,78	0	26 666,67	0	0
5	0	-3840	-9600	0	1003 840	-9600	0	-1000 000	0	0	0
6	0	9600	16 000	26 666,67	-9600	85 333,33	-26 666,67	0	26 666,67	0	0
7	0	0	0	-17 777,78	0	-26 666,67	375 866,67	-178 249,40	-358 088,89	-178 249,40	2 385,14
8	0	0	0	0	-1000 000	0	178 249,40	-4 770,28	-178 249,40	-90 714,79	-4 770,28
9	0	0	0	26 666,67	0	26 666,67	-24 281,53	77 184,73	-2 385,14	4 770,28	11925,70
10	0	0	0	0	0	0	-358 088,89	-178 249,40	358 088,89	178 249,40	-2 385,14
11	0	0	0	0	0	0	-178 249,40	-90 714,79	178 249,40	90 714,79	4 770,28
12	0	0	0	0	0	0	2 385,14	-4 770,28	-2 385,14	4 770,28	23 851,39

* Δ _G	=	F _G	+	R _G
1	0	0	1	R1
2	0	-8,32	2	R2
3	-0,002	-12,8	3	R3
4	Δ4	27,5926	4	0
5	Δ5	-87,4535	5	0
6	Δ6	3,4222	6	0
7	Δ7	7,4074	7	0
8	Δ8	-45,0880	8	0
9	Δ9	29,0966	9	0
10	0	0	10	R10
11	0	-33,5410	11	R11
12	Δ12	-33,5410	12	0

K _G - Estrutura											
4	5	6	7	8	9	10	11	12	1	2	3
4	617 777,78	0	26 666,67	-17 777,78	0	26 666,67	0	-600 000	0	0	0
5	0	1003 840	-9 600	0	-1000 000	0	0	-3 840	-9 600	0	0
6	26 666,67	-9 600	85 333,33	-26 666,67	0	26 666,67	0	9 600	16 000	0	0
7	-17 777,78	0	-26 666,67	375 866,67	178 249,40	-24 281,53	2 385,14	0	0	-358 088,89	-178 249,40
8	0	-1000 000	0	178 249,40	1090 714,79	-4 770,28	-4 770,28	0	0	-178 249,40	-90 714,79
9	26 666,67	0	26 666,67	-24 281,53	-4 770,28	77 184,73	11925,70	0	0	-2 385,14	4 770,28
10	0	0	0	2 385,14	-4 770,28	11925,70	23 851,39	0	0	-2 385,14	4 770,28
11	-600 000	0	0	0	0	0	0	600 000	0	0	0
12	0	-3 840	9 600	0	0	0	0	0	9 600	0	0
13	0	-9 600	16 000	0	0	0	0	0	9 600	32 000	0
14	0	0	0	-358 088,89	-178 249,40	-2 385,14	-2 385,14	0	0	358 088,89	178 249,40
15	0	0	0	-178 249,40	-90 714,79	178 249,40	4 770,28	0	0	178 249,40	90 714,79

Δ _G	=	F _G	+	R _G
4	Δ4	27,5926	4	0
5	Δ5	-87,4535	5	0
6	Δ6	3,4222	6	0
7	Δ7	7,4074	7	0
8	Δ8	-45,0880	8	0
9	Δ9	29,0966	9	0
10	Δ10	-33,5410	10	0
11	0	0	11	R1
12	0	-8,32	12	R2
13	-0,002	-12,8	13	R3
14	0	0	14	R10
15	0	-33,5410	15	R11

TEORIA DE ESTRUTURAS

ISABEL ALVIM TELES

K_{LL}											
4	5	6	7	8	9	12					
617 777.78	0	26 666.67	-17 777.78	0	26 666.67	0	Δ_4	4	-600 000	0	0
0	1003 840	-9 600	0	-1000 000	0	0	Δ_5	5	0	-3 840	0
26 666.67	-9 600	85 333.33	-26 666.67	0	26 666.67	0	Δ_6	6	0	9 600	16 000
-17 777.78	0	-26 666.67	375 866.67	178 249.40	-24 281.53	2 385.14	Δ_7	7	0	0	0
0	-1000 000	0	178 249.40	1090 714.79	-4 770.28	-4 770.28	Δ_8	8	0	0	-358 088.89
26 666.67	0	26 666.67	-24 281.53	-4 770.28	77 184.73	11925.70	Δ_9	9	0	0	-178 249.40
0	0	0	2 385.14	-4 770.28	11925.70	23 851.39	Δ_{12}	12	0	0	-2 385.14

$\Delta_4 = 2.0706E-04$ m
$\Delta_5 = -2.3775E-02$ m
$\Delta_6 = 8.9026E-05$ rad
$\Delta_7 = 1.1614E-02$ m
$\Delta_8 = -2.3761E-02$ m
$\Delta_9 = 3.8915E-03$ rad
$\Delta_{12} = -9.2656E-03$ rad

K_{FL}											
4	5	6	7	8	9	12					
-600 000	0	0	0	0	0	0	Δ_{LL}	4	2.0706E-04	1	600 000
0	-3 840	9 600	0	0	0	0		5	-2.3775E-02	2	0
0	-9 600	16 000	0	0	0	0		6	8.9026E-05	3	0
0	0	0	-358 088.89	-178 249.40	-2 385.14	-2 385.14		7	1.1614E-02	10	0
0	0	0	-178 249.40	-90 714.79	4 770.28	4 770.28		8	-2.3761E-02	11	0

$R_1 = -124.239$ kN
$R_2 = 81.271$ kN
$R_3 = 178.466$ kNm
$R_8 = 89.239$ kN
$R_{10} = 93.131$ kN

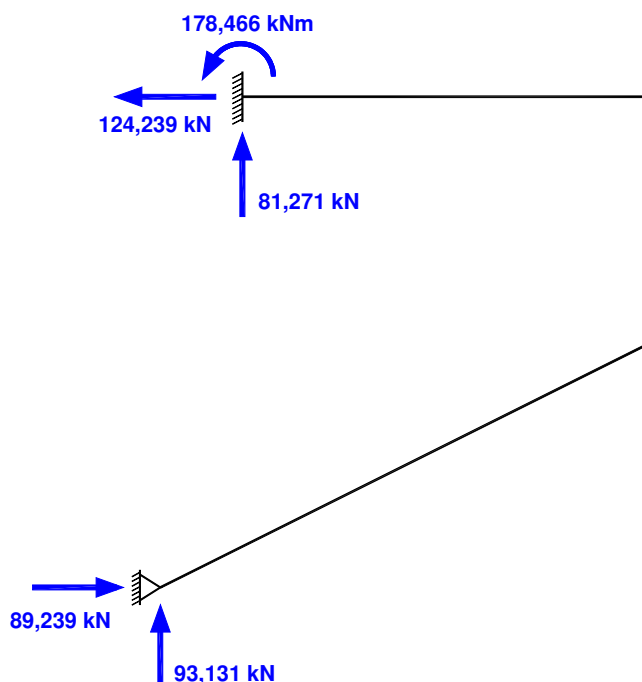
Deslocamentos dos nós da estrutura

$\Delta 1 = 0$
$\Delta 2 = 0$
$\Delta 3 = -2,000 \times 10^{-3} \text{ rad}$ ↺
$\Delta 4 = 2,071 \times 10^{-4} \text{ m}$ →
$\Delta 5 = -2,378 \times 10^{-2} \text{ m}$ ↓
$\Delta 6 = 8,903 \times 10^{-5} \text{ rad}$ ↺
$\Delta 7 = 1,161 \times 10^{-2} \text{ m}$ →
$\Delta 8 = -2,376 \times 10^{-2} \text{ m}$ ↓
$\Delta 9 = 3,892 \times 10^{-3} \text{ rad}$ ↺
$\Delta 10 = 0$
$\Delta 11 = 0$
$\Delta 12 = -9,266 \times 10^{-3} \text{ rad}$ ↺

ALÍNEA b)

Reacções nos apoios

$R1 = H1 = -124,239 \text{ kN}$ ←
$R2 = V1 = 81,271 \text{ kN}$ ↑
$R3 = M1 = 178,466 \text{ kNm}$ ↺
$R10 = H4 = 89,239 \text{ kN}$ →
$R11 = V4 = 93,131 \text{ kN}$ ↑



ALÍNEA c)

Esforços nas barras

$$[F_L] = [K_L] \cdot [\Delta_L] - [F_{L-EN}]$$

$$[\Delta_L] = [T] \cdot [\Delta_G]$$

$$[F_L] = [K_L] \cdot [T] \cdot [\Delta_G] - [F_{L-EN}]$$

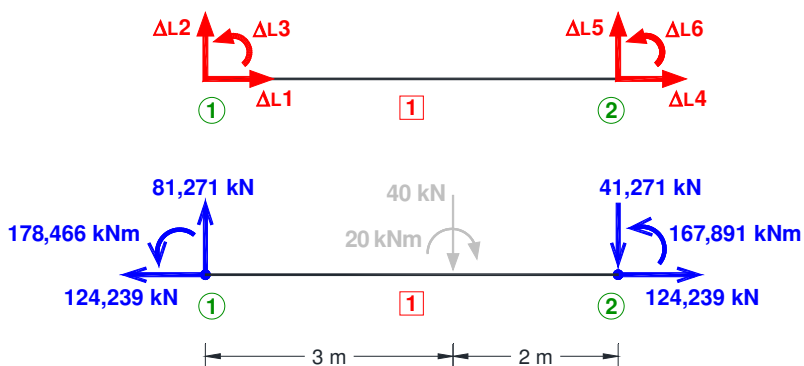
BARRA 1 Nós 1 - 2

$$\begin{aligned} \Delta L1 &\Leftrightarrow \Delta G1 = 0 \\ \Delta L2 &\Leftrightarrow \Delta G2 = 0 \\ \Delta L3 &\Leftrightarrow \Delta G3 = -2,000 \times 10^{-3} \\ \Delta L4 &\Leftrightarrow \Delta G4 = 2,071 \times 10^{-4} \\ \Delta L5 &\Leftrightarrow \Delta G5 = -2,378 \times 10^{-2} \\ \Delta L6 &\Leftrightarrow \Delta G6 = 8,903 \times 10^{-5} \end{aligned}$$

F_L	=	K_L						*	Δ_L	-	F_{L-EN}
F_1		600 000	0	0	-600 000	0	0	1	0	1	0
F_2		0	3 840	9 600	0	-3 840	9 600	2	0	2	-8,32
F_3		0	9 600	32 000	0	-9 600	16 000	3	$-2,000 \times 10^{-3}$	3	-12,8
F_4		-600 000	0	0	600 000	0	0	4	$2,071 \times 10^{-4}$	4	0
F_5		0	-3840	-9 600	0	3840	-9 600	5	$-2,378 \times 10^{-2}$	5	-31,68
F_6		0	9 600	16 000	0	-9 600	32 000	6	$8,903 \times 10^{-5}$	6	31,2

Barra 1 - Nós 1 - 2
 F_L - LOCAL

F_1	-124,239 kN
F_2	81,271 kN
F_3	178,466 Nm
F_4	124,239 kN
F_5	-41,271 kN
F_6	167,891 Nm



TEORIA DE ESTRUTURAS

ISABEL ALVIM TELES

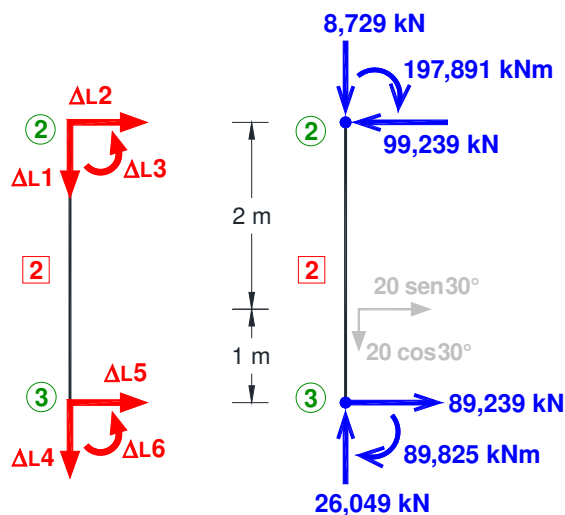
BARRA 2 Nós 2 - 3

$$\begin{aligned}\Delta L1 &\Leftrightarrow -\Delta G5 = 2,378 \times 10^{-2} \\ \Delta L2 &\Leftrightarrow -\Delta G4 = 2,071 \times 10^{-4} \\ \Delta L3 &\Leftrightarrow \Delta G6 = 8,903 \times 10^{-5} \\ \Delta L4 &\Leftrightarrow -\Delta G8 = 2,376 \times 10^{-2} \\ \Delta L5 &\Leftrightarrow -\Delta G7 = 1,161 \times 10^{-2} \\ \Delta L6 &\Leftrightarrow \Delta G9 = 3,891 \times 10^{-3}\end{aligned}$$

F_L	=	K_L	*	Δ_L	-	F_{L-EN}
F_1		1000 000		0		0
F_2		0		17 777,78		26 666,67
F_3		0		26 666,67		53 333,33
F_4		-1000 000		0		0
F_5		0		-17 777,78		-26 666,67
F_6		0		26 666,67		53 333,33

Barra 2 - Nós 2 - 3
 F_L - LOCAL

F_1	8,729 kN
F_2	-99,239 kN
F_3	-197,891 Nm
F_4	-26,049 kN
F_5	89,239 kN
F_6	-89,825 Nm

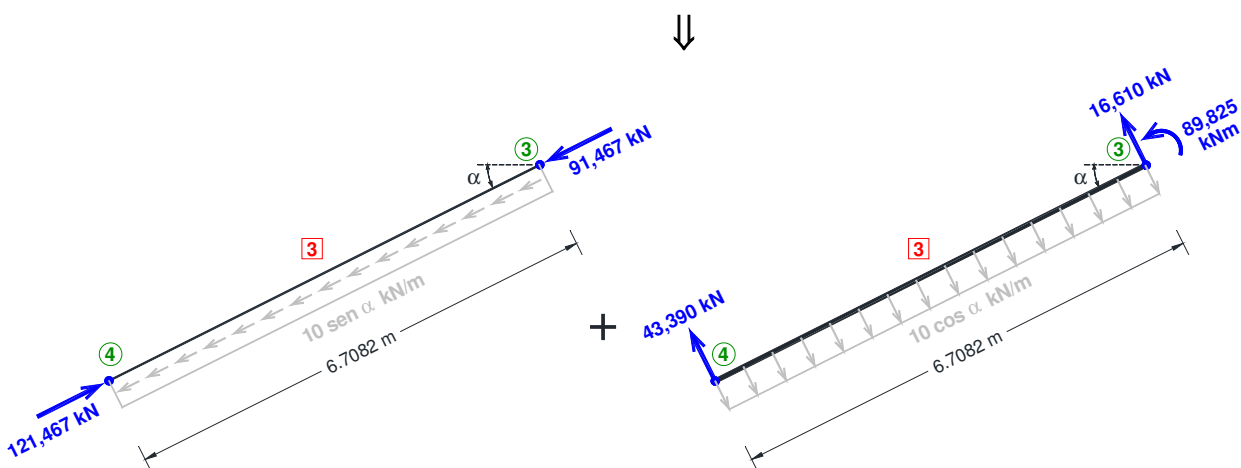
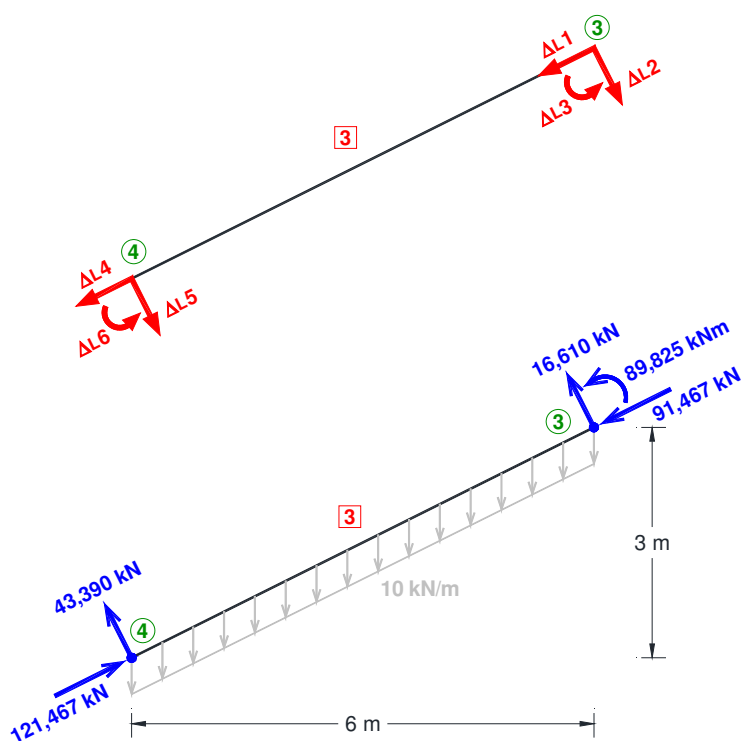


BARRA 3 Nós 3 - 4

F_L	=	K_L						*	T						*	Δ_G	-	F_{L-EN}
F_1		447 213,60	0	0	-447 213,60	0	0		-0,8944	-0,4472	0	0	0	0	7	1,161E-02	1	15
F_2		0	1 590,09	5 333,33	0	-1 590,09	5 333,33		0,4472	-0,8944	0	0	0	0	8	-2,376E-02	2	30
F_3		0	5 333,33	23 851,39	0	-5 333,33	11 925,70		0	0	1	0	0	0	9	3,891E-03	3	33,5410
F_4		-447 213,60	0	0	447 213,60	0	0		0	0	0	-0,8944	-0,4472	0	10	0	4	15
F_5		0	-1 590,09	-5 333,33	0	1 590,09	-5 333,33		0	0	0	0,4472	-0,8944	0	11	0	5	30
F_6		0	5 333,33	11 925,70	0	-5 333,33	23 851,39		0	0	0	0	0	1	12	-9,266E-03	6	-33,5410

Barra 3 - Nós 3 - 4
F_L - LOCAL

F_1	=	91,467 kN
F_2		-16,610 kN
F_3		89,825 Nm
F_4		-121,467 kN
F_5		-43,390 kN
F_6		0 Nm



Diagramas de esforços

